

for *Structural Steel Buildings Allowable Stress Design and Plastic Design*, Chicago, IL, 1989.

16. Zandonini, R., "Semi-Rigid Composite Joints," *Strength and Stability Series, Vol. 8, Connections*, ed. R. Narayanan, Elsevier, London, 1989, pp. 63–120.
17. Easterling, W. S., Gibbings, D. R. and Murray, T. M., "Strength of Shear Studs in Steel Deck on Composite Beams and Joists," *AISC Engineering Journal*, 2nd Qtr., Vol. 30, 1993, pp. 44–55.

18. American Institute of Steel Construction, *Load and Resistance Factor Design Specification for Structural Steel Buildings*, Chicago, IL., 1986.

19. Leon, R. T. and Ammerman, D. J., "Semi-Rigid Composite Connections for Gravity Loads," *AISC Engineering Journal*, 1st Qtr., Vol. 27, 1990, pp. 1–11.

CORRECTIONS

A Practical Approach to the "Leaning" Column

Paper by LOUIS F. GESCHWINDNER
(4th Quarter 1994)

The following corrections should be made in the paper as published. On page 146, second column, the equation should be

$$K_n^2 = \frac{I_i}{57} \pi^2 \frac{207 + 0.0724(57)}{2.40I_i} = 15.23$$

In Figure 11, the spring stiffness in terms of G should be as follows

$$K = \frac{6EI_c}{G_B L_c}$$

This correction in stiffness yields a different deflection in Example 1, page 147, line 11 where $\Delta_{oh} = 2.1307$. The results

of this change and correcting the value of ΣQ in the equation as published yields, from Equation 16

$$K_i = \pi \sqrt{\frac{20,000(238)}{(16(12))^3} \frac{207}{57} \frac{2.1307}{5} \left[1 + \frac{0.216(207 - 105)}{207} \right]} = 3.972$$

and from Equation 17, $K_i = 3.942$. Carrying out the correction for spring stiffness in Example 3, page 149, column 1, yields $\Delta_{oh} = 1.096$ and from Equation 16, $K_1 = 2.166$ and $K_2 = 1.028$ while Equation 17 yields $K_1 = 2.130$ and $K_2 = 1.011$.

The author wishes to thank those readers who took the time to communicate some of these corrections.