

# Predesigned Bolted Framing Angle Connections

AMERICAN INSTITUTE OF STEEL CONSTRUCTION

The current AISC Specification provisions governing the use of mechanical fasteners in structural applications are largely based upon rational rules provided in *Guide to Design Criteria for Bolted and Riveted Joints* by Fisher and Struik.<sup>1</sup> This publication was sponsored by the Research Council on Structural Connections to evaluate and distill the results of voluminous research in this field that has been conducted since the late 1800's.

Unfortunately, adoption of rules which provide the designer with an improved basis for understanding and estimating the real strength of structural connections has made connection design more complex. There are several modes of failure, any one of which may control the strength of a connection, and each must be considered. Thus, the design or checking of a given connection has become more demanding than in the past, when Specification provisions masked the true performance of connections behind overly conservative rules which addressed only a few possible modes of failure. For a complete framing angle connection design, the following must be considered: bolt shear, bearing on connected material, edge distance, block shear, net section shear, connection length, bolt spacing, slip resistance, hole size and shape and, in some cases, other special factors.

Because many typical connections are used repeatedly, in the same project or in different projects, AISC has developed tables of predesigned framing angle connections to reduce the time and cost that would be required for design, review, and checking for approval if unique designs were prepared for each application.

## PREDESIGNED CONNECTION CAPACITIES (TABLES 2, 3, 4, AND 5)

The typical connection is illustrated in Fig. 1. The following controlling variables were used in calculating the tabulated capacities, considering all potential factors affecting connection strength listed in the previous paragraph:

1. Minimum edge and end distance:  $1\frac{1}{4}$  in.
2. Beams: 36 ksi and 50 ksi ( $F_u = 65$  ksi) steel
3. Two connection angles: 36 ksi steel
4. Bolts:  $\frac{3}{4}$ -in. dia. and  $\frac{7}{8}$ -in. dia., A325
5. Bolt spacing: 3 in.
6. Uncoped and single-coped beams
7. W-shapes with nominal web thicknesses up to and including  $\frac{15}{16}$ -in.
8. Bearing connections with bolt threads included (N) or excluded (X) from the shear plane

Note that tables do not apply to friction-type connections, single-angle connections, double-coped beams, nor A307 and A490 bolts insofar as bolt shear capacities are concerned.

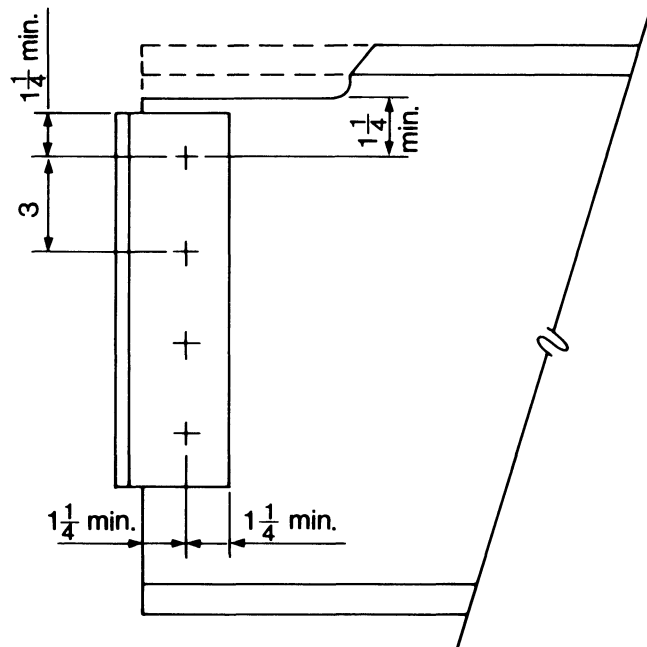


Fig. 1. Dimensions shown are basis for calculation of tabular values for double-angle predesigned connections.

**Table 1. Coefficients for Computing Actual Web Shear Capacity**

No. of Bolts	Uncoped Connections				Coped Connections			
	36 ksi Steel		50 ksi Steel		36 ksi Steel		50 ksi Steel	
	$\frac{3}{4}$ " Bolts	$\frac{7}{8}$ " Bolts	$\frac{3}{4}$ " Bolts	$\frac{7}{8}$ " Bolts	$\frac{3}{4}$ " Bolts	$\frac{7}{8}$ " Bolts	$\frac{3}{4}$ " Bolts	$\frac{7}{8}$ " Bolts
2	72.5	72.5	81.3	81.3	72.5	72.1	81.3	80.8
3	108.8	108.8	121.9	121.9	108.0	108.0	121.9	121.0
4	145.0	145.0	162.5	162.5	145.0	143.9	162.5	161.3
5	181.3	181.3	203.1	203.1	181.3	179.8	203.1	201.5
6	217.5	217.5	243.8	243.8	217.5	215.7	243.8	241.7
7	253.8	253.8	284.4	284.4	253.8	251.6	284.4	282.0
8	290.0	290.0	325.0	325.0	290.0	287.5	325.0	322.2
9	326.3	326.3	365.6	365.6	326.3	323.4	365.6	362.4
10	362.5	362.5	406.3	406.3	362.5	359.2	406.3	402.6

- Notes: 1. Based on dimensions shown in Fig. 1, where coefficient times actual web thickness = connection capacity (where web shear controls).  
 2. Minimum connection angle thicknesses tabulated in Tables 2 through 5 may not be adequate when connection capacities are determined by use of coefficients.  
 3. Tabular values for 50 ksi steel are based on  $F_u = 65$  ksi.

The predesigned connections covered by the tables in this paper are not to be construed as standard connections. They are connections for beam shapes which have been grouped to facilitate presentation in concise tabular form. The calculated capacities are based on the actual web thickness of the critical shape within each group. Thus, the capacities reflect less than the true allowable capacity for shapes in a group other than the critical shape. The tables should not preclude the use of higher capacities for these or similar connections, if supported by calculations in accordance with Specification provisions, using actual dimensions of the beam shapes, connection materials, and fasteners.

For the purposes of table organization, the shapes are grouped in columns according to nominal web thickness and number of fasteners. The connection angles have a length not less than one-half the beam depth nor more than the depth of the beam web clear of the flange-to-web fillets.

The tabulated connection capacities are based on the shape with the thinnest web in the group, indicated by an asterisk. Thus, for a connection using  $\frac{3}{4}$ -in. dia. bolts for an A36 steel beam (Table 2) with a nominal web thickness of  $\frac{3}{16}$ -in., the controlling shape for a 2 bolt group is a W8x10. The capacity for the group was calculated using the actual web thickness for this shape (0.170 in.) and is listed as 12.3 kips. The tabulated capacities, therefore, provide additional conservatism for beams in the groups which have actual web thickness greater than the critical shape.

For beam groups with thick webs, particularly those of 50 ksi steel, bolt shear may govern. For those cases, the capacities are listed with either an N or X prefix, and the shapes for connections which could be governed by bolt shear are included within a shaded area. Two different shadings have been used to distinguish between those cases which may be governed by either beam web thickness or

bolt shear capacity if threads are included in the shear plane (N), and those cases where the capacity is independent of beam web thickness and is entirely dependent on bolt shear capacity (N) or (X). For example, a 2 bolt connection using  $\frac{3}{4}$ -in. dia. bolts with threads included in the shear plane of an A36 steel beam, and nominal web thickness greater than  $\frac{1}{2}$ -in. would have a capacity of 37.2 kips (Table 2). If threads are excluded from the shear plane, the web thickness would control up to a nominal web thickness of  $\frac{1}{16}$ -in., beyond which the maximum 2 bolt capacity would be limited to 53 kips, the maximum shear strength of the bolts.

The tabular capacities are applicable to both uncoped and single-coped connections. In Tables 2 and 4 ( $\frac{3}{4}$ -in. dia. bolts), all tabulated web capacities are governed by end distance and are identical for coped and uncoped connections. In Tables 3 and 5, tabulated web capacities are based on coped connections governed by block shear. In no case are these values more than 1% less than the capacities for equivalent uncoped connections.

#### COEFFICIENTS FOR EXACT CAPACITY CALCULATIONS

The user may encounter cases in which the tabulated capacity (based on the thinnest web in the group) may be slightly less than required for a particular situation. To provide a convenient means for determining the fully justified capacity in accordance with Specification requirements, Table 1 lists coefficients which can be multiplied by the exact web thickness to yield the actual capacity. Values are listed for both the uncoped and single-coped case. Since these slightly larger values may exceed the bolt shear capacity, the capacity calculated using Table coefficients should be checked against the N or X capacities in Tables 2, 3, 4 and 5, as appropriate.

### CONNECTION ANGLE THICKNESSES

The minimum connection angle thicknesses shown at the top of each vertical column in Tables 2, 3, 4 and 5 are those which provide a net area shear strength equal to or greater than the corresponding web capacity tabulated for each group in that column. Thus, in some cases the tabulated minimum angle thickness may be larger than actually required for an individual group within a column. The listed angle thickness is always conservative for the tabulated capacities. However, if a higher capacity is calculated using Table 1 coefficients, the connection angle thickness should be checked.

Listed connection angle thicknesses are valid for the outstanding leg as long as each angle contains the same number of bolts as does the leg connected to the beam web. The detailer should be careful regarding tightening clearances if the gage is small. In such cases, it may be necessary to stagger the bolts in the two legs.

### MINIMUM DIMENSIONS

The minimum dimensions shown in Fig. 1 are the dimensions used strictly for the purpose of calculating the tabulated capacities; they are not intended to restrict the use of larger end distances. For example, usual practice in dimensioning framing angle connections is to provide a setback from the back of the outstanding legs of angles to

allow for possible overrun tolerance on the cut length of the beam. This setback normally results in an end distance greater than the  $1\frac{1}{4}$  in. used in the calculations.

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### REFERENCES

1. *Fisher, J. W. and J. H. A. Struik* Guide to Design Criteria for Bolted and Riveted Joints *John Wiley & Sons, New York, N.Y., 1974.*

**Table 2. Predesigned Bolted Shear Connection Capacities (kips) for Single Coped and Uncoped W Shapes**

Nominal Web Thickness, In.	$\frac{3}{16}$	$\frac{1}{4}$	$\frac{5}{16}$	$\frac{3}{8}$	$\frac{7}{16}$	
Min. Connection Angle Thickness, In.	$\frac{1}{4}$	$\frac{1}{4}$	$\frac{1}{4}$	$\frac{1}{4}$	$\frac{1}{4}$	
No. of Bolts	2	12.3 kips W8x10* W10x12 W12x14	16.0 kips W8x13-24 W10x15-26 W12x16*-30	20.7 kips W8x28*-35 W10x30-39, 49 W12x35-45	25.4 kips W8x40, 48 W10x45*, 54 W12x50-65	30.5 kips W10x60* W12x72
	3	21.8 kips W12x14	23.9 kips W12x16*-30 W14x22-30 W16x26, 31	31.0 kips W12x35-45 W14x34*-48 W16x36, 40 W18x35, 40	37.5 kips W12x50, 53*-65 W14x53, 61 W16x45*, 50, 67 W18x46, 50, 55	45.1 kips W12x72 W14x68*, 74, 90 W16x57, 77 W18x60*, 65, 76
	4		36.3 kips W16x26*, 31	42.8 kips W16x36*, 40 W18x35, 40	50.0 kips W16x45*, 50, 67 W18x46, 50, 55	60.2 kips W16x57, 77 W18x60*, 65, 76
	5			54.4 kips W18x35*, 40	63.4 kips W18x46, 50, 55 W21x44*-62 W24x55	75.2 kips W18x60*, 65, 76 W21x68, 73 W24x62*-76 W27x84
	6				76.1 kips W21x44*-62 W24x55	90.3 kips W21x68, 73 W24x62, 68*, 76 W27x84
	7				100.2 kips W24x55*	105.3 kips W24x62, 68*, 76 W27x84
	8					133.4 kips W27x84
	9					
	10					

Notes: 1. Tabulated connection capacities are based on the shape with the thinnest web in each group, except that capacities in shaded areas may be limited by bolt shear (N or X).

2. In Tables 2 and 4 ( $\frac{3}{4}$ -in. bolts), the connection capacities of coped and uncoped beams are identical.

3. Increased capacities may be computed using actual web thickness for shapes without asterisks, using the coefficients in Table 1.

Beam: 36 ksi  
 Angles: 36 ksi  
 Bolts: A325, 3/4-in.

1/2	5/16	5/8	1 1/16	3/4	13/16-15/16
5/16	5/16	3/8	3/8	7/16	1/2
34.1 kips N-37.2 kips W8x58 W10x68*, 77 W12x79*, 87	39.9 kips N-37.2 kips W8x67 W12x96*	43.9 kips N-37.2 kips W10x88* W12x106	49.3 kips N-37.2 kips W10x100* W12x120	N-37.2 kips X-53.0 kips W10x112	N-37.2 kips X-53.0 kips W12x136-170
51.1 kips N-55.8 kips W12x79*, 87 W14x82, 99, 109 W16x89 W18x71.86	58.2 kips N-55.8 kips W12x96 W14x120 W16x100 W18x97*, 106	66.3 kips N-55.8 kips W12x106* W14x132 W18x119	74.0 kips N-55.8 kips W12x120 W14x145*	N-55.8 kips X-79.5 kips W14x159	N-55.8 kips X-79.5 kips W12x136-170 W14x176, 193
69.6 kips N-74.4 kips W16x89 W18x71, 86* W21x101, 83	77.6 kips N-74.4 kips W16x100 W18x97*, 106 W21x93, 111	87.0 kips N-74.4 kips W18x119 W21x122*, 132		104.4 kips N- 74.4 kips W21x147	
85.2 kips N-93.0 kips W18x71, 86 W21x101, 83 W24x84*, 104, 94 W27x94, 102	97.0 kips N-93.0 kips W18x97*, 106 W21x93, 111 W24x117 W27x114	108.8 kips N- 93.0 kips W18x119 W21x122*, 132 W24x131, 146 W27x146	119.6 kips N- 93.0 kips W24x162 W27x161*	130.5 kips N- 93.0 kips W21x147* W27x178	
102.2 kips N-111.6 kips W21x101, 83 W24x84*, 104, 94 W27x94, 102 W30x99	118.5 kips N-111.6 kips W21x93, 111 W24x117 W27x114 W30x108*-124 W33x118, 130	130.5 kips N-111.6 kips W21x122*, 132 W24x131, 146 W27x146 W30x132, 173 W33x141, 152 W36x135-160	143.6 kips N-111.6 kips W24x162 W27x161* W30x191 W33x201 W36x170	156.6 kips N-111.6 kips X-159.0 kips W21x147* W27x178 W30x211 W33x221 W36x182, 194, 230	N-111.6 kips X-159.0 kips W33x241 W36x210, 245-300
119.3 kips N-130.2 kips W24x84*, 104, 94 W27x94, 102 W30x99	138.3 kips N-130.2 kips W24x117 W27x114 W30x108*-124 W33x118, 130	152.3 kips N-130.2 kips W24x131, 146 W27x146 W30x132, 173 W33x141, 152 W36x130*-160	167.5 kips N-130.2 kips W24x162 W27x161* W30x191 W33x201 W36x170	184.0 kips N-130.2 kips X-185.5 kips W27x178* W30x211 W33x221 W36x182*, 194, 230	N-130.2 kips X-185.5 kips W33x241 W36x210, 245-300
142.1 kips N-148.8 kips W27x94*, 102 W30x99	158.1 kips N-148.8 kips W27x114 W30x108*-124 W33x118, 130	174.0 kips N-148.8 kips W27x146 W30x132, 173 W33x141, 152 W36x135*-160	191.4 kips N-148.8 kips W27x161* W30x191 W33x201 W36x170	210.3 kips N-148.8 kips X-212.0 kips W27x178* W30x211 W33x221 W36x182*, 194, 230	N-148.8 kips X-212.0 kips W33x241 W36x210, 245-300
	179.4 kips N-167.4 kips W33x118*, 130	195.8 kips N-167.4 kips W33x141, 152 W36x135*-160	221.9 kips N-167.4 kips W33x201 W36x170*	236.5 kips N-167.4 kips X-238.5 kips W33x221 W36x182*, 194, 230	N-167.4 kips X-238.5 kips W33x241 W36x210, 245-300
		217.5 kips N-186.0 kips W36x135*-160	246.5 kips N-186.0 kips W36x170	262.8 kips N-186.0 kips X-265.0 kips W36x182*, 194, 230	N-186.0 kips X-265.0 kips W36x210, 245-300

\* Indicates the shape with the least web thickness of all shapes within the group.  
 □ Connection capacity controlled by bolt shear if bolt threads included in shear plane.  
 ■ Connection capacity controlled by bolt shear in all cases.

**Table 3. Predesigned Bolted Shear Connection Capacities (kips) for Single Coped and Uncoped W Shapes**

Nominal Web Thickness, In.	$\frac{3}{16}$	$\frac{1}{4}$	$\frac{5}{16}$	$\frac{3}{8}$	$\frac{7}{16}$	
Min. Connection Angle Thickness, In.	$\frac{1}{4}$	$\frac{1}{4}$	$\frac{1}{4}$	$\frac{1}{4}$	$\frac{1}{4}$	
No. of Bolts	2	12.3 kips W8x10* W10x12 W12x14	15.9 kips W8x13-24 W10x15-26 W12x16*-30	20.5 kips W8x28*-35 W10x30-39, 49 W12x35-45	25.2 kips W8x40, 48 W10x45*, 54 W12x50-65	30.3 kips W10x60* W12x72
	3	21.6 kips W12x14	23.8 kips W12x16*-30 W14x22-30 W16x26, 31	30.8 kips W12x35-45 W14x34*-48 W16x36, 40 W18x35, 40	37.3 kips W12x50, 53*-65 W14x53, 61 W16x45*, 50, 67 W18x46, 50, 55	44.8 kips W12x72 W14x68*, 74, 90 W16x57, 77 W18x60*, 65, 76
	4		36.0 kips W16x26*, 31	42.5 kips W16x36*, 40 W18x35, 40	49.6 kips W16x45*, 50, 67 W18x46, 50, 55	59.7 kips W16x57, 77 W18x60*, 65, 76
	5			53.9 kips W18x35*, 40	62.9 kips W18x46, 50, 55 W21x44*-62 W24x55	74.6 kips W18x60*, 65, 76 W21x68, 73 W24x62, 68*, 76 W27x84
	6				75.5 kips W21x44*-62 W24x55	89.5 kips W21x68, 73 W24x62, 68*, 76 W27x84
	7				99.4 kips W24x55	104.4 kips W24x62, 68*, 76 W27x84
	8					132.3 kips W27x84
	9					
	10					

Notes: 1. Tabulated connection capacities are based on the shape with the thinnest web in each group, except that capacities in shaded areas may be limited by bolt shear (N or X).

2. In Tables 3 and 5 ( $\frac{7}{8}$ -in. bolts), the connection capacities are based on coped beams. When used for uncoped beams, the values for the controlling shapes may be conservative by no more than 1%.

3. Increased capacities may be computed using actual web thickness for shapes without asterisks, using the coefficients in Table 1.

Beam: 36 ksi  
 Angles: 36 ksi  
 Bolts: A325, 7/8-in.

1/2	5/16	5/8	11/16	3/4	13/16-15/16
5/16	3/8	3/8	7/16	7/16	1/2
33.9 kips W8x58 W10x68*, 77 W12x79*, 87	39.7 kips W8x67 W12x96*	43.6 kips W10x88* W12x106	49.0 kips N-50.6 kips W10x100* W12x120	54.4 kips N-50.6 kips W10x112	57.0 kips N-50.6 kips W12x136*-170
50.8 kips W12x79*, 87 W14x82, 99, 109 W16x89 W18x71, 86	57.8 kips W12x96 W14x120 W16x100 W18x97*, 106	65.9 kips W12x106* W14x132 W18x119	73.4 kips N-75.9 kips W12x120 W14x145*	80.5 kips N-75.9 kips W14x159	85.3 kips N-75.9 kips W12x136*-170
69.1 kips W16x89 W18x71, 86* W21x83, 101	77.0 kips W16x100 W18x97*, 106 W21x93, 111	86.3 kips W18x119 W21x122*, 132		103.6 kips N-101.2 kips W21x147	
84.5 kips W18x71, 86 W21x83, 101 W24x84*-104 W27x94, 102	96.2 kips W18x97*, 106 W21x93, 111 W24x117 W27x114	107.9 kips W18x119 W21x122*, 132 W24x131, 146 W27x146	118.7 kips N-126.5 kips W24x162 W27x161*	129.5 kips N-126.5 kips W21x147* W27x178	
101.4 kips W21x83, 101 W24x84*-104 W27x94, 102 W30x99	117.6 kips W21x93, 111 W24x117 W27x114 W30x108*-124 W33x118, 130	129.4 kips W21x122*, 132 W24x131, 146 W27x146 W30x132, 173 W33x141, 152 W36x135*-160	142.4 kips N-151.8 kips W24x162 W27x161* W30x191 W33x201 W36x170	155.3 kips N-151.8 kips W21x147* W27x178 W30x211 W33x221 W36x182, 194, 230	172.6 kips N-151.8 kips W33x241 W36x210, 245*-300
118.2 kips W24x84*-104 W27x94, 102 W30x99	137.1 kips W24x117 W27x114 W30x108*-124 W33x118, 130	151.0 kips W24x131, 146 W27x146 W30x132, 173 W33x141, 152 W36x135*-160	166.1 kips N-177.1 kips W24x162 W27x161* W30x191 W33x201 W36x170	182.4 kips N-177.1 kips W27x178* W30x211 W33x221 W36x182*, 194, 230	201.3 kips N-177.1 kips W33x241 W36x210, 245*-300
140.9 kips W27x94*, 102 W30x99	156.7 kips W27x114 W30x108*-124 W33x118, 130	172.5 kips W27x146 W30x132, 173 W33x141, 152 W36x135*-160	189.8 kips N-202.4 kips W27x161* W30x191 W33x201 W36x170	208.4 kips N-202.4 kips W27x178* W30x211 W33x221 W36x182*, 194, 230	230.0 kips N-202.4 kips W33x241 W36x210, 245*-300
	177.9 kips W33x118*, 130	194.0 kips W33x141, 152 W36x135*-160	219.9 kips N-227.7 kips W33x201 W36x170*	234.5 kips N-227.7 kips W33x221 W36x182*	258.7 kips N-227.7 kips W33x241 W36x210, 245*-300
		215.5 kips W36x135*-160	244.3 kips W36x170	260.4 kips N-253.0 kips W36x182	287.4 kips N-253.0 kips W36x210, 245*-300

\* Indicates the shape with the least web thickness of all shapes within the group.  
 Connection capacity controlled by bolt shear if bolt threads included in shear plane.

**Table 4. Predesigned Bolted Shear Connection Capacities (kips) for Single Coped and Uncoped W Shapes**

Nominal Web Thickness, In.		$\frac{3}{16}$	$\frac{1}{4}$	$\frac{5}{16}$	$\frac{3}{8}$	$\frac{7}{16}$
Min. Connection Angle Thickness, In.		$\frac{1}{4}$	$\frac{1}{4}$	$\frac{1}{4}$	$\frac{1}{4}$	$\frac{5}{16}$
No. of Bolts	2	13.8 kips W8x10* W10x12 W12x14	17.9 kips W8x13-24 W10x15-26 W12x16*-30	23.2 kips W8x28*-35 W10x30-39, 49 W12x35-45	28.4 kips W8x40, 48 W10x45*, 54 W12x50-65	34.1 kips W10x60* W12x72
	3	24.4 kips W12x14	26.8 kips W12x16*-30 W14x22-30 W16x26, 31	34.7 kips W12x35-45 W14x34*-48 W16x36, 40 W18x35, 40	42.0 kips W12x50, 53-65 W14x53, 61 W16x45*, 50, 67 W18x46, 50, 55	50.6 kips W12x72 W14x68*, 74, 90 W16x57, 77 W18x60*, 65, 76
	4		40.6 kips W16x26*, 31	47.9 kips W16x36*, 40 W18x35, 40	56.1 kips W16x45*, 50, 67 W18x46, 50, 55	67.4 kips W16x57, 77 W18x60*, 65, 76
	5			60.9 kips W18x35*, 40	71.1 kips W18x46, 50, 55 W21x44*-62 W24x55	84.3 kips N-93.0 kips W18x60*, 65, 76 W21x68, 73 W24x62, 68*, 76 W27x84
	6				85.3 kips W21x44*-62 W24x55	101.2 kips N-111.6 kips W21x68, 73 W24x62, 68*, 76 W27x84
	7				112.3 kips W24x55	118.0 kips N-130.2 kips W24x62, 68*, 76 W27x84
	8					149.5 kips N-148.8 kips W27x84
	9					
	10					

- Notes: 1. Tabulated connection capacities are based on the shape with the thinnest web in each group, except that capacities in shaded areas may be limited by bolt shear (N or X).  
 2. In Tables 2 and 4 ( $\frac{3}{4}$ -in. bolts), the connection capacities of coped and uncoped beams are identical.  
 3. Increased capacities may be computed using actual web thickness for shapes without asterisks, using the coefficients in Table 1.

Beam: 50 ksi  
 Angles: 36 ksi  
 Bolts: A325, 3/4-in.

1/2	9/16	5/8	11/16	3/4	13/16-15/16
5/16	3/8	7/16	7/16	7/16	7/16
38.2 kips N-37.2 kips W8x58 W10x68*-77 W12x79*, 87	44.7 kips N-37.2 kips W8x67 W12x96*	49.2 kips N-37.2 kips W10x88* W12x106	N-37.2 kips X-53.0 kips W10x100 W12x136		
57.3 kips N-55.8 kips W12x79*, 87 W14x82, 99, 109 W16x89 W18x71, 86	65.2 kips N-55.8 kips W12x96 W14x120 W16x100 W18x97*, 106	74.3 kips N-55.8 kips X-79.5 kips W12x106* W14x132 W18x119	N-55.8 kips X-79.5 kips W12x120 W14x145	N-55.8 kips X-79.5 kips W14x159	
78.0 kips N-74.4 kips W16x89 W18x71, 86* W21x83, 101	86.9 kips N-74.4 kips W16x100 W18x97*, 106 W21x93, 111	97.5 kips N-74.4 kips X-106.0 kips W18x119 W21x122*, 132		N-74.4 kips X-106.0 kips W21x147	
95.5 kips N-93.0 kips W18x71, 86 W21x83, 101 W24x84*-104 W27x94, 102	108.7 kips N- 93.0 kips W18x97*, 106 W21x93, 111 W24x117 W27x114	121.9 kips N- 93.0 kips W18x119 W21x122*, 132 W24x131, 146 W27x146	N- 93.0 kips X-132.5 kips W24x162 W27x161	N- 93.0 kips X-132.5 kips W21x147 W27x178	
114.6 kips N-111.6 kips W21x83, 101 W24x84*-104 W27x94, 102 W30x99	132.8 kips N-111.6 kips W21x93, 111 W24x117 W27x114 W30x108*-124 W33x118, 130	146.3 kips N-111.6 kips X-159.0 kips W21x122*, 132 W24x131, 146 W27x146 W30x132, 173 W33x141, 152 W36x135*, 150, 160	N-111.6 kips X-159.0 kips W24x162 W27x161 W30x191 W33x201 W36x170	N-111.6 kips X-159.0 kips W21x147 W27x178 W30x211 W33x221 W36x182, 194, 230	N-111.6 kips X-159.0 kips W33x241 W36x210, 245-300
133.7 kips N-130.2 kips W24x84*-104 W27x94, 102 W30x99	155.0 kips N-130.2 kips W24x117 W27x114 W30x108*-124 W33x118, 130	170.6 kips N-130.2 kips X-185.5 kips W24x131, 146 W27x146 W30x132, 173 W33x141, 152 W36x135*, 150, 160	N-130.2 kips X-185.5 kips W24x162 W27x161 W30x191 W33x201 W36x170	N-130.2 kips X-185.5 kips W27x178 W30x211 W33x221 W36x182, 194, 230	N-130.2 kips X-185.5 kips W33x241 W36x210, 245-300
159.3 kips N-148.8 kips W27x94*, 102 W30x99	177.1 kips N-148.8 kips W27x114 W30x108*-124 W33x118, 130	195.0 kips N-148.8 kips X-212.0 kips W27x146 W30x132, 173 W33x141, 152 W36x135*-160	N-148.8 kips X-212.0 kips W27x161 W30x191 W33x201 W36x170	N-148.8 kips X-212.0 kips W27x178 W30x211 W33x221 W36x182, 194, 230	N-148.8 kips X-212.0 kips W33x241 W36x210, 245-300
	201.1 kips N-167.4 kips W33x118*, 130	219.4 kips N-167.4 kips W33x141, 152 W36x135*-160	N-167.4 kips X-238.5 kips W33x201 W36x170	N-167.4 kips X-238.5 kips W33x221 W36x182	N-167.4 kips X-238.5 kips W33x241 W36x210, 245-300
		243.8 kips N-186.0 kips W36x135*-160	N-186.0 kips X-265.0 kips W36x170	N-186.0 kips X-265.0 kips W36x182	N-186.0 kips X-265.0 kips W36x210, 245-300

\* Indicates the shape with the least web thickness of all shapes within the group.  
 □ Connection capacity controlled by bolt shear if bolt threads included in shear plane.  
 ■ Connection capacity controlled by bolt shear in all cases.

**Table 5. Predesigned Bolted Shear Connection Capacities (kips) for Single Coped and Uncoped W Shapes**

Nominal Web Thickness, In.		$\frac{3}{16}$	$\frac{1}{4}$	$\frac{5}{16}$	$\frac{3}{8}$	$\frac{7}{16}$
Min. Connection Angle Thickness, In.		$\frac{1}{4}$	$\frac{1}{4}$	$\frac{1}{4}$	$\frac{1}{4}$	$\frac{5}{16}$
No. of Bolts, <i>n</i>	2	13.7 kips W8x10* W10x12 W12x14	17.8 kips W8x13-24 W10x15-26 W12x16*-30	23.0 kips W8x28*-35 W10x30-39, 49 W12x35-45	28.3 kips W8x40, 48 W10x45*, 54 W12x50-65	33.9 kips W10x60* W12x72
	3	24.2 kips W12x14	26.6 kips W12x16*-30 W14x22-30 W16x26, 31	34.5 kips W12x35-45 W14x34*-48 W16x36, 40 W18x35, 40	41.7 kips W12x50, 53*, 58, 65 W14x53, 61 W16x45*, 50, 67 W18x46, 50, 55	50.2 kips W12x72 W14x68*, 74, 90 W16x57, 77 W18x60*, 65, 76
	4		40.3 kips W16x26*, 31	47.6 kips W16x36*, 40 W18x35, 40	55.6 kips W16x45*, 50, 67 W18x46, 50, 55	66.9 kips W16x57, 77 W18x60*, 65, 76
	5			60.5 kips W18x35*, 40	70.5 kips W18x46, 50, 55 W21x44*-62 W24x55	83.6 kips W18x60*, 65, 76 W21x68, 73 W24x62, 68*, 76 W27x84
	6				84.6 kips W21x44*-62 W24x55	100.3 kips W21x68, 73 W24x62, 68*, 76 W27x84
	7				111.4 kips W24x55	117.0 kips W24x62, 68*, 76 W27x84
	8					148.2 kips W27x84
	9					
	10					

- Notes: 1. Tabulated connection capacities are based on the shape with the thinnest web in each group, except that capacities in shaded areas may be limited by bolt shear (N or X).
2. In Tables 3 and 5 ( $\frac{7}{8}$ -in. bolts), the connection capacities are based on coped beams. When used for uncoped beams, the values for the controlling shapes may be conservative by no more than 1%.
3. Increased capacities may be computed using actual web thickness for shapes without asterisks, using the coefficients in Table 1.

Beam: 50 ksi  
Angles: 36 ksi  
Bolts: A325, 7/8-in.

1/2	9/16	5/8	11/16	3/4	13/16-15/16
5/16	3/8	7/16	7/16	1/2	1/2
38.0 kips W8x58 W10x68*-77 W12x79*, 87	44.4 kips W8x67 W12x96*	48.9 kips W10x88* W12x106	54.9 kips N-50.6 kips W10x100* W12x136		
56.9 kips W12x79*, 87 W14x82, 99, 109 W16x89 W18x71, 86	64.7 kips W12x96 W14x120 W16x100 W18x97*, 106	73.8 kips N-75.9 kips W12x106* W14x132 W18x119	82.3 kips N-75.9 kips W12x120 W14x145*	90.1 kips N-75.9 kips W14x159	
77.4 kips W16x89 W18x71, 86* W21x83, 101	86.3 kips W16x100 W18x97*, 106 W21x93, 111	96.8 kips N-101.2 kips W18x119 W21x122*, 132		116.1 kips N-101.2 kips W21x147	
94.7 kips W18x71, 86 W21x83, 101 W24x84*-104 W27x94, 102	107.8 kips W18x97*, 106 W21x93, 111 W24x117 W27x114	120.9 kips N-126.5 kips W18x119 W21x122*, 132 W24x131, 146 W27x146	133.0 kips N-126.5 kips W24x162 W27x161*	145.1 kips N-126.5 kips W21x147* W27x178	
113.6 kips W21x83, 101 W24x84*-104 W27x94, 102 W30x99	131.7 kips W21x93, 111 W24x117 W27x114 W30x108*-124 W33x118, 130	145.0 kips N-151.8 kips W21x122*, 132 W24x131, 146 W27x146 W30x132, 173 W33x141, 152 W36x135*, 150, 160	159.5 kips N-151.8 kips W24x162 W27x161* W30x191 W33x201 W36x170	174.0 kips N-151.8 kips W21x147* W27x178 W30x211 W33x221 W36x182, 194, 230	193.4 kips N-151.8 kips W33x241 W36x210, 245*-300
132.5 kips W24x84*-104 W27x94, 102 W30x99	153.7 kips W24x117 W27x114 W30x108*-124 W33x118, 130	169.2 kips N-177.1 kips W24x131, 146 W27x146 W30x132, 173 W33x141, 152 W36x135*, 150, 160	186.1 kips N-177.1 kips W24x162 W27x161* W30x191 W33x201 W36x170	204.5 kips N-177.1 kips W27x178* W30x211 W33x221 W36x182*, 194, 230	225.6 kips N-177.1 kips W33x241 W36x210, 245*-300
157.9 kips W27x94*, 102 W30x99	175.6 kips W27x114 W30x108*-124 W33x118, 130	193.3 kips N-202.4 kips W27x146 W30x132, 173 W33x141, 152 W36x135*, 150, 160	212.7 kips N-202.4 kips W27x161* W30x191 W33x201 W36x170	233.6 kips N-202.4 kips W27x178* W30x211 W33x221 W36x182*, 194, 230	257.8 kips N-202.4 kips W33x241 W36x210, 245*-300
	199.3 kips W33x118*, 130	217.4 kips N-227.7 kips W33x141, 152 W36x135*, 150, 160	246.4 kips N-227.7 kips W33x201 W36x170*	262.7 kips N-227.7 kips W33x221 W36x182*	289.9 kips N-227.7 kips W33x241 W36x210, 245*-300
		241.6 kips N-253.0 kips W36x135*, 150, 160	273.8 kips N-253.0 kips W36x170	291.9 kips N-253.0 kips W36x182	322.1 kips N-253.0 kips W36x210, 245*-300

\* Indicates the shape with the least web thickness of all shapes within the group.

☐ Connection capacity controlled by bolt shear if bolt threads included in shear plane.